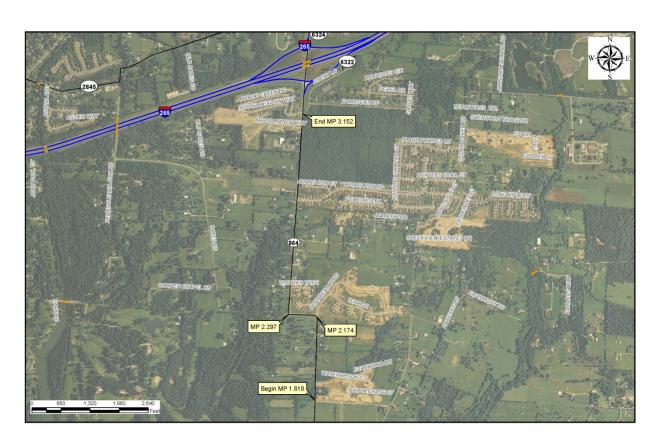
APPENDIX A MAP



PROJECT LOCATION MAP

APPENDIX B PROJECT AUTHORIZATION

PROJECT AUTHORIZATION **AUTHORIZATION NO: 863130** It is hereby ordered that the project herein described be undertaken and accomplished within the funding level authorized Project Id Project Id Number Federal District County 6 Yrp Item Number Project No. HWY ADD **JEFFERSON** 05-00481 STPM 8776 029 056 8640 01-004 05 TYPE OF PROJECT ROUTE NUMBER FACILTY NAME SYSTEMS BEULAH CHURCH 032 - RECONST W/ADD LNS KY 864 ROAD SCOPE OF PROJECT PROJECT LENGTH KY 864 - WIDEN BEULAH CHURCH ROAD FROM 2 TO 3 LANES FROM I-265 TO CEDAR CREEK ROAD. 1.547 MI **6 YR PLAN ITEM PARENT NUMBER** NUMBER OF BRIDGES PROGRAM PRIORITY RS ITEM NUMBER 5-00965.12-2012 UTILITIES PLANNING DESIGN RIGHT OF WAY PROJECT PHASE DOH DOH AND CONSTRUCTION TITLE DEEDED TO: MAINTENANCE OTHER RESPONSIBILITY PARTICIPATING AGENCIES **FUNDING & TIME ACCOUNTABILITY** LOCAL FEDERAL **FHWA** STATE OTHER REQUESTED FUNDS FOR THIS AUTHORIZATION ITEM NUMBER PHASE FUND **PROGRAM** FISCAL YEAR FEDL APPR. **ENACTED 6YR** % DIFFERENCE CURRENT SUFFIX CODE PLAN AMOUNT VS 6YP AMT **FUNDING FEDERAL** STATE REQUEST 05-00481.00 D 700,000 12 FD52 2012 2012 L230 Current KD Date **Current Funding Request** 700,000 4/5/2012 **Estimate** Total Approved by **AUTHORIZATION SUMMARY FOR THIS 10-1 SERIES** TOTAL AUTHORIZATION INITIAL CURRENT PROJECT **PHASE PROJECT ESTIMATE ESTIMATE** TO DATE (INCL. CURRENT REQUEST) 700,000 \$ 700,000 Design \$ 700,000 Total \$ 700,000 \$ 700,000 \$ 700,000 REMARKS: THIS AUTHORIZATION PROVIDES INITIAL DESIGN FUNDS TO BEGIN THE DESIGN PHASE OF THE PROJECT. DE.

Signed and Approved by:

4/11/2012

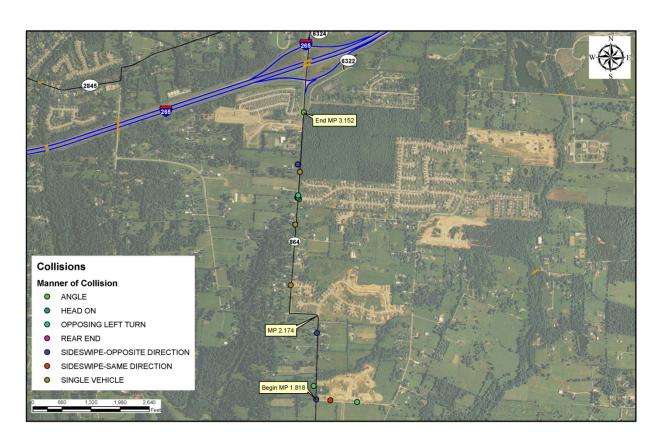
Project Approval Recommended By:

4/10/2012

APPENDIX C

CRASH DATA

ROADWAY	LATITUDE	LATITUDE LONGITUDE MILEPOINT DATE	MILEPOINT		INJURED WI	'EATHER	WEATHER ROAD CONDITION	MANNER OF COLLISION	LIGHT CONDITION
COOPER CHAPEL	38.1047	-85.6142	1.877	2/25/2010	0 CF	0 CLOUDY	DRY	ANGLE	DAYLIGHT
COOPER CHAPEL	38.1079	-85.6139	2.1	5/1/2009	10 CF	0 сгоиру	WET	SIDESWIPE-OPPOSITE DIRECTION	DARK-HWY LIGHTED/OFF
BEULAH CHURCH	38.1109	-85.6160	2.416	11/8/2010	1 CL	CLEAR	DRY	SINGLE VEHICLE	DUSK
BEULAH CHURCH	38.1136	-85.6157	2.599	7/9/2011	10 CF	0 CLEAR	DRY	REAR END	DAYLIGHT
BEULAH CHURCH	38.1147	-85.6156	2.672	3/5/2009	0 CF	CLEAR	DRY	SINGLE VEHICLE	DAYLIGHT
BEULAH CHURCH	38.1163	-85.6153	2.78	2/4/2009	0 CLEAR	.EAR	ICE	ANGLE	DAYLIGHT
BEULAH CHURCH	38.1164	-85.6155	2.785	1/5/2009	10 CF	0 CLEAR	DRY	SINGLE VEHICLE	DARK-HWY LIGHTED/ON
BEULAH CHURCH	38.1164	-85.6154	2.785	5/20/2009	2 CL	CLEAR	DRY	HEAD ON	DARK-HWY LIGHTED/OFF
BEULAH CHURCH	38.1165	-85.6154	2.795	10/1/2009	10 CF	0 сгоиру	DRY	OPPOSING LEFT TURN	DAYLIGHT
BEULAH CHURCH	38.1180	-85.6153	2.897	6/24/2009	0 CF	0 CLEAR	DRY	SINGLE VEHICLE	DARK-HWY NOT LIGHTED
BEULAH CHURCH	38.1184	-85.6154	2.929	7/2/2010	O CF	0 CLEAR	DRY	SIDESWIPE-OPPOSITE DIRECTION	DAYLIGHT
BEULAH CHURCH	38.1217	-85.6149	3.154	2/4/2009	0 CLEAR	EAR	DRY	ANGLE	DAYLIGHT
CEDAR CREEK	38.1038	-85.6129		4/30/2009	0 CF	0 CLEAR	DRY	SIDESWIPE-SAME DIRECTION	DAYLIGHT
CEDAR CREEK	38.1038	-85.6140	0.001	2/19/2009	1 CL	CLOUDY	DRY	SIDESWIPE-OPPOSITE DIRECTION	DARK-HWY NOT LIGHTED
CEDAR CREEK	38.1037	-85.6108	0.181	10/1/2011	O CF	0 CLOUDY	DRY	OPPOSING LEFT TURN	DAYLIGHT



COLLISION LOCATIONS

APPENDIX D KYTC'S COMMON GEOMETRIC PRACTICE GUIDELINES

(13)

COMMON GEOMETRIC PRACTICES URBAN ROADWAYS (OTHER THAN FREEWAYS)

		URBAN	LOCAL S	TREETS	URBAN C	OLL	ECTOF	R STRE	ETS	URBAN A	RTE	ERIAL	STRE	ETS
DESIGN S	SPEED (14)	20 M	I.P.H 30 N	I.P.H.		MIN.	30 M.P.I	H.		30	M.P.F	l 60 M	l.P.H.	
NUMBER	OF LANES		MINIMUM 2			MIN	IIMUM 2)		MIN	IIMUM 2	. (4	i)
LANE	RESIDENTIAL		MIN, 10'	(1)		MIN,		2		12' FRE	E EL	ow co	NDITIO	v (2)
WIDTH	COMMERCIAL		MIN. 11'			MIN.				11' MIN. INTERI				
	INDUSTRIAL		MIN. 12'	(3)		MIN.	-	(3)		II MIIN, INTERI	TOF	ED FLOY	V CONDI	TION
SIDEWALK	RESIDENTIAL COMMERCIAL						MINIM! DESIRA	THE PARTY NAMED IN COLUMN	16					
WIDTH OF	EAR ROADWAY NEW AND (11) CTED BRIDGES				MINIM	IUM C	URB TO	CURB W	/IDTH					
BERM	IAREA (5)						10' TYF	PICAL						
MINIMUM RA	ADIUS (FEET)						(6							
MAXIMUM GRADE		- R) - MAX. 15% - C) - MAX. 8%			M.P.H.	30	35 4	0 45	50	(9) M.P.H.	30	35 40	45 50	55 60
				12)	LEVEL		9	8	7	LEVEL	8	7	6	5
(PERC	JENI)		IAX. 8%		ROLLING	11	10	9	8	ROLLING	9	8	7	6
		22			MOUNTAIN		12	11	10	MOUNTAIN	11	10	9	8
	PAVEMENT 8 SLOPE				R	ATE O	F CROS	S SLOPE	E = 2%	6				
	SHOULDER S SLOPE	65 ge		EARTH - 8	1%					PAVED	-4%	6		
SUPEREL	LEVATION	(10)	4% MAX.			49	6 MAX.				4% -	6% MAX	(.	
MINIMUM ST	TOPPING (7)	M.P.H.	20	25	30		35	40)	45	50	5	5	60
SIGHT DISTAN	TOPPING NCE (FEET)	MIN.	115	155	200		250	30	5	360	125	49	95	570

- R) = RESIDENTIAL

- C) = COMMERCIAL

- I) = INDUSTRIAL

- 1 TURNING LANES: 9' MINIMUM 12' DESIRABLE; PARKING LANES: RESIDENTIAL 7' MINIMUM 10' DESIRABLE; COMMERCIAL & INDUSTRIAL 9' MINIMUM 12' DESIRABLE.
- (2) TURNING LANES: 10' MINIMUM 12' DESIRABLE; PARKING LANES: 9' MINIMUM 12' DESIRABLE.
- (3) VERTICAL CURBS WITH HEIGHTS OF 6" OR GREATER ADJACENT TO TRAVELED WAY SHOULD BE OFFSET A MINIMUM OF 1 FOOT. WHEN A CURB AND GUTTER SECTION IS PROVIDED, THE GUTTER PAN WIDTH, NORMALLY 2 FEET, SHOULD BE USED AS THE OFFSET DISTANCE.
- THE NUMBER OF LANES TO BE PROVIDED ON STREETS WITH A CURRENT ADT OF 2000 OR GREATER SHOULD BE DETERMINED BY A HIGHWAY CAPACITY ANALYSIS OF THE DESIGN TRAFFIC VOLUMES. SUCH ANALYSIS SHOULD BE MADE FOR FUTURE DESIGN TRAFFIC. (DESIRABLE)
- (5) THE BERM AREA IS TYPICALLY FROM FACE OF CURB TO 2 FEET BEHIND BACK OF SIDEWALK.
- (6) REFER TO CHAPTER 3 OF AASHTO'S "A POLICY ON GEOMETRIC DESIGN OF HIGHWAYS AND STREETS" CURRENT EDITION.
- MINIMUM STOPPING SIGHT DISTANCES ARE BASED ON HEIGHT OF EYE 3.5 FT. & HEIGHT OF OBJECT OF 2.0 FT. BOTH HORIZONTAL & VERTICAL ALIGNMENTS CONSIDERED.
- (8) NORMAL PAVEMENT CROSS SLOPES ON BRIDGES SHALL BE 2 PERCENT.
- ARTERIALS WITH LARGE NUMBERS OF TRUCKS AND OPERATING NEAR CAPACITY SHOULD CONSIDER GRADES FLATTER THAN THOSE IN RURAL SECTIONS TO AVOID UNDESIRABLE REDUCTIONS IN SPEEDS.
- (10) SUPERELEVATION MAY NOT BE REQUIRED ON LOCAL STREETS IN RESIDENTIAL AND COMMERCIAL AREAS.
- THE BRIDGE WIDTH FOR URBAN ROADWAYS WITH SHOULDERS AND NO CURBS SHOULD NOT BE LESS THAN WIDTHS SHOWN FOR RURAL ROADS APPROVED ROADWAY WIDTHS.
- (12) MAXIMUM GRADES OF SHORT LENGTHS (LESS THAN 500') AND ON ONE-WAY DOWN GRADES MAY BE ONE PERCENT STEEPER.
- (3) FOR GUIDANCE ON FREEWAYS, REFER TO AASHTO, "A POLICY ON GEOMETRIC DESIGN OF HIGHWAYS AND STREETS."
- 14 INTERMEDIATE DESIGN SPEEDS (5 M.P.H. INCREMENTS) MAY BE APPROPRIATE WHERE TERRAIN AND OTHER ENVIRONMENTAL CONDITIONS DICTATE.
- (5) REFER TO AASHTO'S "GUIDE FOR THE DEVELOPMENT OF BICYCLY FACILITIES", CURRENT EDITION, WHEN COMBINING A PEDESTRIAN SIDEWALK WITH A BICYCLE PATH.

APPENDIX E HIGHWAY CAPACITY SOFTWARE ANALYSIS

Phone: Fax: E-Mail: _____Directional Two-Lane Highway Segment Analysis______ Analyst Agency/Co. KYTC Date Performed 3/8/2012 Analysis Time Period Highway KY 864 MP 1.818 to MP 3.082 From/To Jurisdiction Louisville Analysis Year 2012 Description KY 864 widening _____Input Data_____ Peak hour factor, PHF 0.89 Highway class Class 3 Shoulder width 3.0 ft % Trucks and buses 4 3.0
11.0 ft % Trucks crawling
1.3 mi Truck crawl speed 0.0
Recreational vehicles 1
Recreational zones 100 Lane width 0.0 Segment length 0.0 mi/hr Rolling Terrain type % No-passing zones 100 Grade: Length Access point density 25 Up/down % /mi Analysis direction volume, Vd 450 veh/h Opposing direction volume, Vo 300 veh/h ______Average Travel Speed_____ Direction Analysis(d) Opposing (o) PCE for trucks, ET 2.1 1.8 PCE for RVs, ER 1.1 1.1 Heavy-vehicle adj. factor,(note-5) fHV 0.968 0.957 Grade adj. factor,(note-1) fg 0.95 0.86 550 pc/h Directional flow rate, (note-2) vi 410 pc/h Free-Flow Speed from Field Measurement: Field measured speed, (note-3) S FM mi/h Observed total demand, (note-3) V veh/h Estimated Free-Flow Speed: Base free-flow speed, (note-3) BFFS 45.0 mi/h Adj. for lane and shoulder width, (note-3) fLS 3.0 mi/h Adj. for access point density, (note-3) fA 6.3 mi/h Free-flow speed, FFSd 35.8 mi/h 2.7 mi/h Adjustment for no-passing zones, fnp

25.6

71.7

mi/h

Average travel speed, ATSd

Percent Free Flow Speed, PFFS

Percent Time-Spent-Follow	ing		
Direction PCE for trucks, ET PCE for RVs, ER Heavy-vehicle adjustment factor, fHV Grade adjustment factor, (note-1) fg Analysis(d) 1.2 0.992	,	Opposing 1.6 1.0 0.977 0.87	
Directional flow rate, (note-1) vi 531 possible by 531 possibl	51.6 38.3	397	pc/h
Level of Service and Other Perform	ance Mea	sures	
Level of service, LOS Volume to capacity ratio, v/c Peak 15-min vehicle-miles of travel, VMT15 Peak-hour vehicle-miles of travel, VMT60 Peak 15-min total travel time, TT15 Capacity from ATS, CdATS Capacity from PTSF, CdPTSF Directional Capacity	D 0.37 164 585 6.4 1470 1494 2566	veh-mi veh-h veh/h veh/h veh/h	
Passing Lane Analysis			
Total length of analysis segment, Lt Length of two-lane highway upstream of the passing Length of passing lane including tapers, Lpl Average travel speed, ATSd (from above) Percent time-spent-following, PTSFd (from above) Level of service, LOSd (from above)	lane, L	1.3 u - - 25.6 73.5 D	mi mi mi mi/h
Average Travel Speed with Pass	ing Lane		
Downstream length of two-lane highway within effective length of passing lane for average travel speed Length of two-lane highway downstream of effective	d, Lde	-	mi
<pre>length of the passing lane for average travel a Adj. factor for the effect of passing lane on average speed, fpl Average travel speed including passing lane, ATSpl</pre>		d - - -	mi
Percent Time-Spent-Following with		Lane	
Downstream length of two-lane highway within effective of passing lane for percent time-spent-following Length of two-lane highway downstream of effective	tive len	gth -	mi
the passing lane for percent time-spent-follow Adj. factor for the effect of passing lane on percent time-spent-following, fpl Percent time-spent-following		- -	mi
including passing lane, PTSFpl		-	%
Level of Service and Other Performance Measu:	res with	Passing	Lane
Level of service including passing lane, LOSpl Peak 15-min total travel time, TT15	- -	veh-h	

______ Bicycle Level of Service _____

Posted speed limit, Sp	55
Percent of segment with occupied on-highway parking	0
Pavement rating, P	3
Flow rate in outside lane, vOL	505.6
Effective width of outside lane, We	14.00
Effective speed factor, St	4.79
Bicycle LOS Score, BLOS	4.94
Bicycle LOS	E

- 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific dewngrade segments are treated as level terrain.
- 2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.
- 3. For the analysis direction only and for v>200 veh/h.
- 4. For the analysis direction only.
- 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

Phone: Fax: E-Mail: _____Directional Two-Lane Highway Segment Analysis_____ Analyst Agency/Co. KYTC Date Performed 3/8/2012 Analysis Time Period Highway KY 864 From/To MP 3.082 to MP 3.152 Jurisdiction Louisville Analysis Year 2012 Description _____Input Data_____ Peak hour factor, PHF 0.89 Highway class Class 3 Shoulder width 8.0 ft % Trucks and buses 4 11.0 ft % Trucks crawl speed
0.1 mi Truck crawl speed
Recreational vehi Lane width 0.0 왕 Segment length 0.0 mi/hr Rolling % Recreational vehicles 1 Terrain type % No-passing zones 100 Grade: Length - mi Access point density 25 Up/down 용 /mi Analysis direction volume, Vd 450 veh/h Opposing direction volume, Vo 300 veh/h _____Average Travel Speed____ Direction Analysis(d) Opposing (o) PCE for trucks, ET 2.1 1.8 PCE for RVs, ER 1.1 1.1 Heavy-vehicle adj. factor,(note-5) fHV 0.968 0.957 Grade adj. factor,(note-1) fg 0.95 0.86 550 pc/h Directional flow rate, (note-2) vi 410 pc/h Free-Flow Speed from Field Measurement: Field measured speed, (note-3) S FM mi/h Observed total demand, (note-3) V veh/h Estimated Free-Flow Speed: Base free-flow speed, (note-3) BFFS 45.0 mi/h Adj. for lane and shoulder width, (note-3) fLS 0.4 mi/h Adj. for access point density, (note-3) fA 6.3 mi/h Free-flow speed, FFSd mi/h 38.3 2.7 mi/h Adjustment for no-passing zones, fnp

28.2

73.7

mi/h

Average travel speed, ATSd

Percent Free Flow Speed, PFFS

Percent Time-Spent-Follow	ing		
Direction PCE for trucks, ET PCE for RVs, ER Heavy-vehicle adjustment factor, fHV Grade adjustment factor, (note-1) fg Analysis(d) 1.2 0.992		Opposing 1.6 1.0 0.977 0.87	
	c/h 51.6 38.3 73.5	397	pc/h
Level of Service and Other Perform	ance Mea	sures	
Level of service, LOS Volume to capacity ratio, v/c Peak 15-min vehicle-miles of travel, VMT15 Peak-hour vehicle-miles of travel, VMT60 Peak 15-min total travel time, TT15 Capacity from ATS, CdATS Capacity from PTSF, CdPTSF Directional Capacity	D 0.37 13 45 0.5 1470 1494 2566	veh-mi veh-h veh/h veh/h veh/h	
Passing Lane Analysis			
Total length of analysis segment, Lt Length of two-lane highway upstream of the passing Length of passing lane including tapers, Lpl Average travel speed, ATSd (from above) Percent time-spent-following, PTSFd (from above) Level of service, LOSd (from above)	lane, L	0.1 - - 28.2 73.5 D	mi mi mi mi/h
Average Travel Speed with Pass	ing Lane	·	
Downstream length of two-lane highway within effection length of passing lane for average travel speet Length of two-lane highway downstream of effective	d, Lde	-	mi
<pre>length of the passing lane for average travel Adj. factor for the effect of passing lane on average speed, fpl Average travel speed including passing lane, ATSpl</pre>		- -	mi
Percent Time-Spent-Following with		Lane	
Downstream length of two-lane highway within effect of passing lane for percent time-spent-following	tive len ng, Lde	gth -	mi
Length of two-lane highway downstream of effective the passing lane for percent time-spent-follow Adj. factor for the effect of passing lane on percent time-spent-following, fpl Percent time-spent-following		- -	mi
including passing lane, PTSFpl		-	%
Level of Service and Other Performance Measu	res with	Passing	Lane
Level of service including passing lane, LOSpl Peak 15-min total travel time, TT15	- -	veh-h	

______ Bicycle Level of Service _____

Posted speed limit, Sp	55
Percent of segment with occupied on-highway parking	0
Pavement rating, P	3
Flow rate in outside lane, vOL	505.6
Effective width of outside lane, We	27.00
Effective speed factor, St	4.79
Bicycle LOS Score, BLOS	2.27
Bicycle LOS	В

- 1. Note that the adjustment factor for level terrain is 1.00, as level terrain is one of the base conditions. For the purpose of grade adjustment, specific dewngrade segments are treated as level terrain.
- 2. If vi (vd or vo) >= 1,700 pc/h, terminate analysis-the LOS is F.
- 3. For the analysis direction only and for v>200 veh/h.
- 4. For the analysis direction only.
- 5. Use alternative Exhibit 15-14 if some trucks operate at crawl speeds on a specific downgrade.

APPENDIX F PROJECT TEAM MEETING MINUTES

Meeting Minutes – KY 864 Beulah Church Road (Project Team Meeting No. 1)

The first project team meeting for the KY 864 Data Needs Analysis (DNA) Study was held on May 7, 2012 at 10:00 a.m. EST at the District 5 Design conference room in Louisville. The following individuals were in attendance:

Jill Asher KYTC – Central Office Planning

Dane Blackburn KYTC District 5 Planning
Paul Davis KYTC District 5 Design
Keith Downs KYTC District 5 Design

Robert Farley KYTC – Central Office Design
Tom Hall KYTC District 5 Planning

Brian Meade KYTC District 5 Project Development

Mikael Pelfrey KYTC – Central Office Planning

Tala Quinio KYTC District 5 Design

Keith Downs welcomed those in attendance and said the purpose of the meeting was to discuss the KY 864 Beulah Church Road widening project (Item 5-481.00) in Louisville, specifically the DNA study being prepared by Mikael Pelfrey.

Mikael Pelfrey then took over and began going through the aspects in the DNA study. This study will be one of the first completed under the new eight page format. Existing conditions of the project were explained. Project limits are Cedar Creek Road to the south and Rocky Lane to the north. This was modified slightly from the initial project listing of the Gene Snyder Freeway (I-265) to the north, because the existing segment between I-265 and Rocky Lane is three lanes. There are no existing plans available.

The extension of Cooper Chapel Road (Item 5-404.01) is currently in design. This project is within the KY 864 project limits and proposes extended Cooper Chapel Road to the east to eventually intersect with Bardstown Road. It was stated by those in attendance from District 5 this project wasn't high priority, and right-of-way money was being withheld by FHWA until later phases of design were complete.

Each of the nine elements of the project purpose and need were highlighted. The McNeely Lake Master Plan was brought to attention. This plan proposes the addition of a road through the park, which would affect traffic patterns. There aren't a high number of collisions, only 12 along the entire project limits within a three year period, but four were at the intersection with Adams Run Road.

The Preliminary Environmental Overview was completed by Jeff Schaefer, of District 5 environmental.

Three alternatives were developed in addition to the no build. The first called for widening KY 864 from the proposed tie-in at the Cooper Chapel Road extension to Rocky Lane, which would make the most heavily travelled stretch of road from the extension to the Gene Snyder Freeway three lanes. Another alternative focused on the collisions at Adams Run Road and suggested adding a turning lane. The final alternative widened the route to three lanes along the entire project limits.

Several recommendations were then suggested by the district. These are summarized below:

- Consider an alternative to eliminate the 90° curves (later eliminated once it was realized it would be extremely costly due to right-of-way expenses).
- The typical section for Cooper Chapel Road should not be used, but a map should be added showing the location of the proposed project in relation to the widening on KY 864.
- Add a map with projects from the Highway Plan and the Unscheduled Project List.
- Traffic volumes may lead to widening considerations of more than three lanes.
- A sight distance problem at the beginning of the project limits at the intersection with Cedar Creek Road.
- Consider a 10' shared use path on one side of the roadway because of the proximity of McNeely Lake Park to help with bicycle traffic.
- Louisville Metro had a permit to improve some cross drains and fixed headwall. Box culverts were also extended. Ditches were filled in and drainage was piped to avoid steep drop offs.
 No drainage problems to the knowledge of the district.
- Travis Thompson (District 5 design) stopped in for a period. Mr. Thompson lives in the area and stated the three way stops were not a current problem.

Ultimately it was advised three alternatives be incorporated into the final DNA, in addition to the no build. The first alternative would address the sight distance issues at the intersection of KY 864 and Beulah Church Road. The second alternative would widen KY 864 to three lanes from Adams Run Road to Rocky Lane, to help with collisions. The final alternative would widen the route from Cooper Chapel Road to Rocky Lane. It was determined widening along the entire project limits was unnecessary at this time.

Mikael Pelfrey would make the needed modifications and send the DNA to Keith Downs, who would complete the cost estimates for each alternative.

The meeting adjourned at approximately 11:45 EST.

APPENDIX G COST ESTIMATES

Explanation of Estimates Project: 5-0481.00 KY 864 Beulah Church Road DNA Study

<u>PHASE</u>	ALTERNATE 1	ALTERNATE 2	ALTERNATE 3	ALTERNATE 4
DESIGN:	NO BUILD	\$35,000	\$257,000	\$599,000
R/W:	n .	\$84,000	\$624,000	\$1,454,000
UTILITIES:	n .	\$9,000	\$455,000	\$1,335,000
CONST:	n .	<u>\$221,000</u>	\$1,652,000	\$3,848,000
TOTAL	n .	\$349,000	\$2,988,000	\$7,236,000

Alternate #1 - No Build - This alternate should be carried forward, but does not meet the needs identified for the project.

Alternate #2: Spot Improvement at Cedar Creek Road Intersection - There is a T-legged intersection at KY 864 and Cedar Creek Road at the southern study limits (MP 1.818). Currently vehicles traveling southbound do not stop, while those going in the northbound or westbound direction encounter a stop sign. Trim vegetation along KY 864 to provide vehicles on Cedar Creek Road better sight distance before having to make their turning movement.

Alternate #3: Minor widening from Adams Run Road to Rocky Lane - Widen KY 864 from 2 lanes to 3 lanes from Adams Run Road (MP 2.785) to Rocky Lane (MP 3.152), a distance of 0.367 miles. The template should match the existing template at the northern study limits at Rocky Lane -- two 11' driving lanes and a 14' two way center left turn lane. Shoulders at minimum should be 3' but could be up to 8' in width depending on available right-of-way. The typical section should also include a 10' shared use path to accomodate bicyclists from nearby McNeely Lake Park and pedestrians from residential development growth. The largest subdivision utilizes Adams Run Road for access. Conseqently, the intersection of KY 864 and Adams Run Road is the only location within the study limits with much of a crash history. If funding is an issue, this segment should be addressed first. Currently there is only a stop sign requiring vehicles to stop exiting Adams Run Road. In addition to the widening north of Adams Run Road, a 225 ft right turn lane should be constructed south of the intersection on northbound KY 864 to help with rear end crashes.

Alternate #4: Minor widening from Cooper Chapel Road to Rocky Lane - Widen KY 864 from 2 lanes to 3 lanes from Cooper Chapel Road (MP 2.297) to Rocky Lane (MP 3.152), a distance of 0.855 miles. The template should be rural -- two 11' driving lanes, a 14' two way center turn lane, 3' to 8' shoulders (depending on available right-of-way), and a 10' shared use path. This alternative widens the driving route to 3 lanes on KY 864 from the Gene Snyder Freeway (I-265) to the stop controlled intersection at Cooper Chapel Road.

NOTES:
Design Cost:
Estimated on Per Mile basis: \$750,000

Right of Way Cost::

Estimated on Per Mile basis: Ron Geveden, ROW Supervisor, recommended \$1,700,000 per mile

Utility Cost:

Estimated by D5 Utility Section:

Notes:

The Utility poles are PACKED with utility companies on them. But they all appear to be on the ROW line and therefore, we would not have to reimburse.

The estimate appears high for the water and sewer, but it was worst case of total relocation.

The water is close to the edge of road in many areas.

There is a MSD pump station near Cedar Creek Road. All design should avoid (Alt #4). That relocation would add \$500

					cation Form				
Conorol Ir	-f	4:00.		•	st Estimate	10/ 004		4.040	
General Ir			County	JEFFERSON	Route	KY 864	MP		
UNL # or Ite		5-0481	BA - P 2.1	Prepared By:	AKD	DATE:		May 18, 2012	445
Length (Mi.)	0.049		Median wid.	0	# Lanes			Pave. Depth (in.)	14.5
Ex.R/W (Ft.)			NewR/W(Ft.)			Total Width (all lanes)	35	Shoulder Width (each side)	2
Brief Descrip		•						- There is a T-legg	
from Project				at KY 864 and		at the southern	Stuc	dy limits (MP 1.818)).
TOTAL PR		I ESI	IMAIE:		\$ 347,100			• • • • • • •	
Planning:	\$	-		Design:		Right of Way:		\$ 83,300	
Utilities:	\$	9,000		Construction		\$ 220,500			
Construc	tion:	Total	Construct	ion Cost	\$ 220,500				
	~	PerMi	le Average	eCost:	\$4,500,000				
					Total Project	t Cost =		\$ 220,500	
		Itemiz	ed Constr	uction Estir	nate:	(Use Best Avail	able	Information)	
				<u>Quantity</u>	<u>Unit</u>	<u>Unit Price</u>		Total Cost	
		Excava							
		Asphalt							
		DGA Detour							
		Bridge							
		Other							
		Other							
		Other							
		Other							
		Other Other							
			laneous	30	%	\$0		\$ -	
		IVIISCEI	iarieous	30	Total Construction	· · · · · · · · · · · · · · · · · · ·		\$ -	
		* Misce	llaneous cha	rges are a Perg	entage of all other		sted	above.	
				_	ring and Grubbing,	•			
					g, Culvert Pipes, et				
				e in the OTHE	R cell if approximate	te quantities are	kno	wn.	
	CONST		ON						
	COMME								
Decima	and NO		I Daniana (21					
Design:	r		al Design (\$ 34,300	Ф 7 00 000			
		rer Will	e Average D	esign Estimate		\$700,000		¢ 24.000	
		D	1 - 1 0 1 1	-ti Di	Total Design Estin			\$ 34,300	
		Percen	t of Constru	ction, Design		Percent		0	
	550101			Total Design E	stimate (percent) =	=		\$ -	
	DESIGN		MENIS						
	and NO					8			
Planning	:	Tota	al Planning	Cost	\$ -				
		Per Mil	e Average P	lanning Estima	ate:				
					Total Planning Est	timate (mileage)	=	\$ -	
		Percen	t of Design,	Planning Estir	mate	Percent			
				Total Planning	Estimate (percent)) =		\$ -	
	PLANN	ING CO	MMENTS						
	and NO	TES:							

				-	ication Form			
			Pre	eliminary Co	st Estimate			
Right of \	Way:	Total E	Estimated F	2/W Cost	\$ 83,300			
						=		
	Г	Per Mile	e Average E	stimated R/W		\$1,700,000	Φ 00.000	
		Itemize	d Right of W	lav Estimate	Total R/VV Estima	ated Cost (mileage)	= \$ 83,300	
		ItCIIIIZC	a ragin or vi	ay Estimate				
					Quantity	Avg. Value	Total Value	
		Farm A	cres					
		Comme	ercial Acres					
		Non-De	velopable Ac	re				
		# of Ho						
		# of Bui						
			nercials Bldg:	3				
		# of Gra	aves					
		Other						
		Other						
		Other		-10/-f D/M		Φ.		
		Adminis	strative & Leg		-614/	-	Φ.	
					of Way Cost =		\$ -	
		Per Acr	e Average E	stimated R/W		. 10 (()		
					Total R/W Estima	ated Cost (mileage)	=	
		** Diabt	of Move option	natas ara basa	d on boot cooumnt	iona at the time of a	atimata	
	DICUT	-OF-WA		lates are base	d on best assumpt	ions at the time of e	estimate.	
	COMM		Ī					
	and NC							
	and NC	/ I LO.		ROW Supervis	sor: Ron Geveden	Recommended \$1,	700 000 per mile	
				TOW Capolina	Son from Govedon	Trocommonaca wij	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Utilities:		Tota	I Utility Co	ost	\$ 9,000			
			,			<u>-</u>		
		Per M	ile Averag	e Utility Cos	st:	\$0		
			_	Total Utili	ty Estimated Cost	=	\$ -	
		Itemiz	ed Utility I	Estimate				
				Quantity	<u>Unit</u>	<u>Unit Price</u>	Total Cost	
		Gas						
		Power						
		Telepho	one					
		Sewer						
		Water	_					
		Cont &	St. 25%+20%	11	1	9,000	\$ 9,000	
				**Total	Utility Cost =		\$ 9,000	
		44 1411						
	11711.17		estimates ar	e based on be	st assumptions at	the time of estimate).	
	UTILIT	Y	estimates ar	e based on be	st assumptions at	the time of estimate) .	
	COMM	Y ENTS						2.02
		Y ENTS	The Utility po	oles are PACKI	ED with utility com	panies on them. Bu	ut they all appear to be ontingencies, Misc (25	

Alternative 2: Spot Improvement at Cedar Creek Road intersection

There is a T-legged intersection at KY 864 and Cedar Creek Road at the southern study limits (MP 1.818). Currently vehicles traveling southbound do not stop, while those going in the northbound or westbound direction encounter a stop sign. Remove trees and vegetation on the southeast quadrant of the intersection along KY 864 to the south to provide vehicles on Cedar Creek Road better sight distance before having to make their turning movement and if necessary shave the top of bank to achieve adequate sight distance, approximately 260 ft of tree removal.

				_	cation Form				
General Ir	nforma	tion:		JEFFERSON	Route	KY 864	MP	2.785-3152	
UNL # or Ite		5-0481	County	Prepared By:	AKD	DATE:		May 18, 2012	
Length (Mi.)	0.367		Median wid.	0	# Lanes			Pave. Depth (in.)	14.5
Ex.R/W (Ft.)			NewR/W(Ft.)	-		Total Width (all lanes)	35	Shoulder Width (each side)	0
Brief Descrip	otion Sun	nmary	Alternate #3	: Minor widenin	g:Widen KY 864 f		lane	s from Adams Run	
from Project	ID Form		Road (MP 2	.785) to Rocky	Lane (MP 3.152), a	a distance of 0.36	37 m	iles.	
TOTAL PR	ROJEC	T ESTI	MATE:		\$ 2,984,156				
Planning:	\$	-		Design:	\$ 256,900	Right of Way:		\$ 623,900	
Utilities:	\$ 45	51,856		Construction:		\$ 1,651,500			
Construc	tion:	Total	Constructi	on Cost	\$ 1,651,500				
	V		le Average		\$4,500,000				
	- 1	. 0	io / tvoi age	,0001.	Total Projec			\$ 1,651,500	
	Г	Itemiz	ed Constr	uction Estir		(Use Best Availa	able		
					<u>Unit</u>	Unit Price		Total Cost	
		Excavat	tion:	9780	CY	\$15		\$ 146,700	
		Asphalt		6010	Ton	\$75		\$ 450,731	
		DGA		3141	Ton	\$20		\$ 62,829	
		Detour							
		Bridge Sidewal	k	1077	SY	\$40		\$ 43,061	
		Curb &		3876	LF	\$15		\$ 58,133	
		L&W		311	SY	\$70		\$ 21,759	
		Other				·			
		Other							
		Other							
		Other	000000	20	0/	Ф 7 02 24 4		¢ 224.0C4	
		*Miscell	aneous	30	% Total Construction	\$783,214		\$ 234,964 \$ 1,018,178	
	CONST COMMI and NO	This cos Guardra could be RUCTIC ENTS	st might inclu ail, Seeding, S e added abov	de cost of Clea Staking, Striping	entage of all other ring and Grubbing, g, Culvert Pipes, et R cell if approxima	, Mobilization, Detc. Any of these	mob ndivi	ilization, idual cost	
Design:		Tota	l Design C	Cost	\$ 256,900				
				esign Estimate	e:	\$700,000			
					Total Design Estir	mate (mileage) =		\$ 256,900	
		Percent	t of Constru	ction, Design	Estimate	Percent		0	
				Total Design E	stimate (percent) =	=		\$ -	
	DESIGN	N COMM	IENTS		u /				
	and NO	TES:							
Planning			l Planning	Cost	\$ -				
. iaiiiiig				lanning Estima	ate:				
					Total Planning Es	timate (mileage)	=	\$ -	
		Percent	t of Design,	Planning Estir	nate	Percent			
				Total Planning	Estimate (percent)) =		\$ -	
	PLANN and NO		MMENTS						

			Dro	ioot Idontifi	ication Form			
				-	ost Estimate			
Right of \	Nay:	Total E	Estimated R	/W Cost	\$ 623,900	1		
J						_		
	Г	Per Mil	e Average Es	stimated R/W	Cost:	\$1,700,000		
					Total R/W Estima	ated Cost (mileage	e) = \$ 623,900	
		Itemize	d Right of W	ay Estimate				
					Quantity	Avg. Value	Total Value	1
		Farm A	cres		Quartity	Avg. value	Total value	i
			ercial Acres					
			velopable Ac	re				
		# of Ho						
		# of Bui						İ
			nercials Bldgs	3				
		# of Gra						
		Other						
		Other						
		Other						
		Adminis	strative & Leg	al %of R/W		\$ -		
		-		**Total Right	of Way Cost =		\$ -	
		Per Acı	e Average E	stimated R/W	Cost:			
	P. Land				Total R/W Estima	ated Cost (mileage)) =	
								•
				ates are base	d on best assump	tions at the time of	f estimate.	
		·OF-WA	Y					
	COMM							
	and NO	TES:		DOM 0 :	5 0 1	5	4 700 000	
				ROW Supervis	sor: Ron Geveden	Recommended \$	1,700,000 per mile	
Utilities:		Tata		-4	A 454.050			
Otilities.		Tota	I Utility Co	ost	\$ 451,856	<u></u>		
		Per M	ile Average	e Utility Cos		\$0		
					ty Estimated Cost	<u> </u>	\$ -	
		Itemiz	ed Utility E			T		7
			T	Quantity	<u>Unit</u>	<u>Unit Price</u>	<u>Total Cost</u>	
		Gas						
		Power						
		Telepho	ne	1	Lumpaum	120 E2E	¢ 120 525	
		Sewer Water		1 1	Lump sum	138,535 173,090	\$ 138,535 \$ 173,090	
			St. 25%+20%	<u> </u>	Lump sum	140,231	\$ 140,231	
		COIL	JI. ZJ /0+ZU /	· · · · · · · · · · · · · · · · · · ·	Utility Cost =	140,231	\$ 451,856	
				TOtal	Othicy Cost =		Ψ 431,030	J
		** Itility	estimates ar	e hased on he	st assumntions at	the time of estima	te	
	UTILIT			o basea on be	ot accumptions at	the time of estima		
	COMM							
	and NO		The Utility po	oles are PACK	ED with utility com	panies on them. I	But they all appear to	be on
							Contingencies, Misc (
				Engineering (

Alternative 3: Minor widening from Adams Run Road to Rocky Lane

Widen KY 864 from 2 lanes to 3 lanes from Adams Run Road (MP 2.785) to Rocky Lane (MP 3.152), a distance of 0.367 miles. The template should match the existing template at the northern study limits at Rocky Lane -- two 11' driving lanes and a 14' two way center left turn lane. Shoulders at minimum should be 3' but could be up to 8' in width depending on available right-of-way. The typical section should also include a 10' shared use path to accommodate bicyclists from nearby McNeely Lake Park and pedestrians from residential development growth. The largest subdivision utilizes Adams Run Road for access. Consequently, the intersection of KY 864 and Adams Run Road is the only location within the study limits with much of a crash history. If funding is an issue, this segment should be addressed first. Currently there is only a stop sign requiring vehicles to stop exiting Adams Run Road. In addition to the widening north of Adams Run Road, a 225 ft right turn lane should be constructed south of the intersection on northbound KY 864 to help with rear end crashes.

Template: 2' C&G, 10' Shared use path, 5' Sidewalk, 2-11' Lanes and 13' CLTL

Pavement Width: 35' gutter to gutter.

Sidewalk: one side: 5' wide

Embankment: 10,000 CY per Mi x 0.978mi = 9,780 CY

Project Identification Form Preliminary Cost Estimate											
General Information: County			JEFFERSON	Route	KY 864	MP	2.297-3.152				
UNL # or Item # 5-0481			County	Prepared By:	AKD	DATE:		May 21, 2012			
Length (Mi.) 0.855		Median wid.	0	# Lanes			Pave. Depth (in.)	14.5			
Ex.R/W (Ft.) 30			NewR/W(Ft.) 64		" Lario	Total Width (all lanes)	35	Shoulder Width (each side)	0		
Brief Description Summary Alternate #4			: Widen KY 864	from 2 lanes to 3		er C		.297)			
from Project		•	to Rocky Lai	ne (MP 3.152),	a distance of 0.85	5 miles.			,		
TOTAL PR			MATE:		\$ 7,229,694						
Planning:			Design:			Right of Way: \$		\$ 1,453,500			
Utilities:	\$ 1,33	30,194		Construction:		\$ 3,847,500		. , ,			
Construc		•	Constructi	on Cost	\$ 3,847,500	, , ,					
0011011141			le AverageCost:		\$4,500,000						
					Total Projec		\$ 3,847,500				
		Itemiz	ed Constr	uction Estir		(Use Best Availa	able	, ,			
					Unit	Unit Price		Total Cost			
		Excavat	tion:	13340	CY	\$15		\$ 200,100			
		Asphalt		14001	Ton	\$75		\$ 1,050,068			
<u> </u>		DĠA		7319	Ton	\$20		\$ 146,373			
		Detour									
	Bridge Sidew Curb & L & W			5040	0)/			<u>Ф</u> 200 040			
				5016	SY	\$40		\$ 200,640			
			Gutter	724	SY	\$70		\$ 50,693			
O		Other		124	01	Ψισ		Ψ 30,033			
		Other									
		Other									
		Other									
	*Miscella		aneous	30	%	\$1,647,874		\$ 494,362			
	Total Construction Cost = \$ 2,142,2 * Miscellaneous charges are a Percentage of all other major cost not listed above. This cost might include cost of Clearing and Grubbing, Mobilization, Demobilization, Guardrail, Seeding, Staking, Striping, Culvert Pipes, etc. Any of these individual cost could be added above in the OTHER cell if approximate quantities are known.										
	CONST			e in the OTHE	R cell il approxima	te quantities are	KNOV	vn.			
CONSTRUCTION COMMENTS											
	and NO										
Design:		Tota	l Design C	Cost	\$ 598,500						
				esign Estimate	•	\$700,000					
					Total Design Estir	mate (mileage) =		\$ 598,500			
	Percent of Construction, Design Estimate Percent										
	Total Design Estimate (percent) =							\$ -			
	DESIGN	N COMM	IENTS		(1)						
	and NO										
Planning			l Planning	Cost	\$ -						
i iaiiiiiig				lanning Estima	•						
					Total Planning Es	timate (mileage)	=	\$ -			
	Percent of Design, Planning Estimate Percent										
				Total Planning Estimate (percent) =				\$ -			
	PLANN and NO		MMENTS								
	and NO	ILS.									

			Dua	iaat lalamtifi	ingtion Form					
				-	cation Form ost Estimate					
Right of Way:		Total Estimated R/W Cost		\$ 1,453,500	<u> </u>					
		Dor Mil	o Average Es	stimated P/W	Cost	\$1,700,000				
	Per Mile Average Estimated R/W				Total R/W Estimated Cost (mileage) = \$ 1,453,500					
	г	Itemize	d Right of W	ay Estimate			·)	1,100,000		
	-									
					Quantity	Avg. Value	Tota	al Value		
	Farn									
			ercial Acres							
			velopable Ac	re						
		# of Ho								
		# of Bui	nercials Bldgs							
		# of Gra		•						
	Other		1463							
		Other								
		Other								
		Adminis	strative & Leg	al %of R/W		\$ -				
				**Total Right	of Way Cost =		\$	-		
		Per Acı	e Average E	stimated R/W	Cost:				ı	
			_		Total R/W Estima	ated Cost (mileage	e) =			
				ates are base	d on best assump	tions at the time of	estima	ıte.		
	RIGHT-OF-WAY									
	COMMENTS									
	and NC	d NOTES:			sor: Pon Govedon	Recommended \$	1 700 0	100 par mila		
				NOW Supervis	soi. Roii Gevedeii	i Necommended \$	1,700,0	oo per mile		
Utilities:		Tota	I Utility Co	st	\$ 1,330,194					
			,		,					
		Per M	ile Average	e Utility Cos	st:	\$0				
				Total Utili	ty Estimated Cost	:=	\$			
	▼ Itemized Utility Estimate									
				Quantity	<u>Unit</u>	<u>Unit Price</u>		Total Cost		
		Gas								
		Power								
		Telepho	one			222.22		222.225		
		Sewer		1	Lump Sum	386,825	\$	386,825		
		Water	C+ 250/ +200/	<u>1</u>	Lump Sum	530,550	\$	530,550		
		Cont &	St. 25%+20%	•	Itility Coot —	412,819	\$	412,819		
				Total	Utility Cost =		Ф	1,330,194	l	
		** Utility	estimates ar	e based on be	st assumptions at	the time of estima	te.			
	UTILIT		- Cammarao ar		a	or country				
	СОММ									
	and NO	TES:				panies on them. [
						ave to reimburse.	Conting	encies, Misc (25%) +	
		State Forces Engineering (20%)								

Alternative 4: Minor widening from Cooper Chapel Road to Rocky Lane

Widen KY 864 from 2 lanes to 3 lanes from Cooper Chapel Road (MP 2.297) to Rocky Lane (MP 3.152), a distance of 0.855 miles. The template should be rural -- two 11' driving lanes, a 14' two way center turn lane, 3' to 8' shoulders (depending on available right-of-way), and a 10' shared use path. This alternative widens the driving route to 3 lanes on KY 864 from the Gene Snyder Freeway (I-265) to the stop controlled intersection at Cooper Chapel Road. This alternative would tie in to the Cooper Chapel Road extension in Phase II Design (Item No. 5-404.01) as shown in Exhibit 3.

Template: 2' C&G, 10' Shared use path, 5' Sidewalk, 2-11' Lanes and 13' CLTL

Pavement Width: 35' gutter to gutter.

Sidewalk: one side: 5' wide

Embankment: 10,000 CY per Mi x 1.334mi = 13,340 CY

APPENDIX H PROJECT PHOTOS



KY 864 at Cedar Creek Road, heading north



KY 864, heading north



KY 864 at Hornbeam Boulevard, heading north



Hornbeam Boulevard at KY 864 intersection, looking south



KY 864 approaching Cooper Chapel Road/Beulah Church Road intersection, heading west



Cooper Chapel Road at KY 864/Beulah Church Road intersection, looking east



KY 864, north of Trotter Trace, heading north



KY 864, heading north with vertical elevation changes



KY 864 approaching Adams Run Road, heading north



Adams Run Road at KY 864 intersection, looking north



Adams Run Road at KY 864 intersection highlighting sight distance issue, looking south



KY 864, between Adams Run Road and Rocky Lane, heading north



Rocky Lane at KY 864 intersection, looking south



Rocky Lane at KY 864 intersection, looking north